BUILDINGWORK
architecture design preservation

Correction Notice #2: STRUCTURAL January 04, 2021

To	Nouri Samiee-Nejad Seattle DCI 700 5th Ave Suite 2000 PO Box 34019 Seattle, WA 98124 Nouri.Samiee@seattle.gov
Project Address	423 2nd Ave Ext S Seattle WA 98104
Project No.	6508387
Sender	Matt Aalfs, BuildingWork matt@buildingwork.design

Nouri:

Please see below responses to STRUCTURAL Correction Notice #2 dated October 08, 2020. Please note the building has changed ownership and program since the previous submittal, and all drawing sheets reflect these changes.

Per correction #1, This project has been reviewed for conformance with one or more of the following codes: 2012 Seattle Building Code (SBC); 2012 Seattle Existing Building Code (SEBC); 2012 Seattle Energy Code (SEC).

Corrections:

1 Section 1704.2 - Please return the attached SDCI Statement of Structural Special Inspections, signed by the owner or engineer or architect acting as the owner's agent. Note: SDCI will not accept the signature of the contractor on the statement.

Response: The SDCI Statement of Structural Special Inspections form has been uploaded to the project portal as a part of this response.

2 Provide a letter from Geotech engineer indicating that he/she has reviewed foundation plans and that they conform to soil report recommendations.

Response: A letter from the Geotech engineer has been attached to this response.

3. Section 1704.5 - Structural observation is required for this structure. Nominate an architect or structural engineer to perform the observations using the attached SDCI Statement of Structural Special Inspection form. Fill in the name of the nominated firm, sign the schedule, and return it to the SDCI reviewer. Before SDCI issues the Certificate of Occupancy for the building, the structural observer must submit a written statement to SDCI that the site visits have been made, and identifying any deficiencies which, to the best of the structural observer's knowledge, have not been resolved.

Response: The SDCI Statement of Structural Special Inspections form has been uploaded to the project portal as a part of this response.

End of Correction Response

☐ Revised Schedule ☐ Addition to Previous Schedule ☑ 2012 SEBC Prescriptive

SDCI Statement of Structural Observation

Project Number 6508387-CN
Project Address 423 2ND AV ET S
SEATTLE, WA 98104
Architect
Engineer

Date 10/8/2020 10:28:51 AM
SDCI Plan Examiner Nouri Samiee-Nejad
Architect Phone
Engineer Phone

Prior to issuance of a building permit, the owner, architect, or engineer acting on behalf of the owner shall appoint an engineering firm to provide structural observation and shall sign and submit this form to the building official.

Property Owner, Architect, or Engineer Signature
I hereby certify that the engineering firm named below has been engaged to perform structural observation required by the Seattle Building Code. It is the responsibility of the owner or the owner's designee to notify the observer in a timely manner when observation is required.

Signature	Title	Date	Phone
	Architect	January 4, 2020	(206) 775-8671

Structural Observation Firm Name	Structural Observation Firm Phone
Swenson Say Faget	(206) 443-6212

Call (206) 684-8860 to schedule a pre-construction conference before the start of construction

Seattle Department of Construction and Inspections
700 Fifth Ave, Suite 2000, P.O. Box 34019, Seattle, WA 98124-4019

☐ **Revised Schedule** ☐ **Addition to Previous Schedule** ☑ **2012 SEBC Prescriptive**

SDCI Statement of Structural Special Inspection

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Date 10/8/2020 10:28:51 AM
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Architect Phone
Engineer Phone

Prior to issuance of a building permit, the owner, architect, or engineer acting on behalf of the owner shall appoint an inspection agency and shall sign and submit this form to the building official.

Property Owner, Architect, or Engineer Signature
I hereby certify that the engineering firm and inspection agency named below has been engaged to perform the special inspections outlined below as required by the Seattle Building Code. It is the responsibility of the owner or the owner's designee to notify the inspection agency or observer in a timely manner when the inspections listed below are required.

Signature	Title	Date	Phone
	Architect	January 4, 2021	(206) 775-8671

Inspection Agency Name	Inspection Agency Phone
Mayes Testing Engineers, Inc.	(425) 742-9360

Required Special Inspections

	Inspection Type	Description
1.	Cold Formed Steel Framing	
2.	Epoxy Grouting	
3.	Reinforced Concrete - Cip	
4.	Mechanical Anchor Bolt Installation	
5.	Structural Steel Erection	
6.	Structural Steel Fabrication	
7.	Steel Seismic Resistance System	

Call (206) 684-8860 to schedule a pre-construction conference before the start of construction

GeoEngineers
17425 NE Union Hill Road, Suite 250
Redmond, Washington 98052
425.861.6000

December 23, 2020

Satterberg Foundation
c/o Forterra
901 5th Avenue, Suite 2200
Seattle, Washington 98164

Attention: Alison Crowley

Subject: Geotechnical Report Review Letter
Metropole Building Redevelopment
421 2nd Avenue Extension South
Seattle, Washington
File No. 24937-001-00

This letter summarizes our review comments for the geotechnical engineering report prepared by ADAPT Engineering for the Metropole Building redevelopment project located at 421 2nd Avenue Extension South in Seattle, Washington. The report prepared by ADAPT is titled "Geotechnical Engineering Evaluation, Metropole Hotel Renovation, 423 2nd Avenue South Ext, Seattle, Washington 98104" and is dated May 5, 2016. Our services are requested to review the existing report and provide comments or recommendations for potential changes to design recommendations in the existing report, if needed. Our review comments and/or recommendations for the report discussed herein consider the geotechnical elements of the design.

PROJECT DESCRIPTION AND SUBSURFACE CONDITIONS

We understand the project consists of rehabilitation of the historic Metropole Building in Seattle's Pioneer Square neighborhood. The Metropole Building is a multi-story brick building that is currently unoccupied. The property consists of King County Parcel Number 5247800595 and is bounded by Yesler Way to the north, 2nd Avenue South to the east, an adjacent building to the south, and an alley to the west.

Rehabilitation of the building will consist of construction of new stairways, a new elevator, and reconstruction of several elements in the building. Street improvements around the building consisting of new utility and grading work are also planned. We understand that new foundations will be constructed as part of the rehabilitation in the crawl space and basement levels of the building. New foundations will consist of 7-inch-diameter micropiles and 3-inch-diameter steel pipe (pin) piles. Structural improvements will also be completed throughout the building to resist seismic forces.

Subsurface conditions at the site were characterized by ADAPT Engineering by drilling two borings within the northern portion of the existing building. Both borings extended to a depth of 26.5 feet below the existing

ground surface. We also reviewed logs from existing borings completed within the site vicinity. Four soil units are documented at or near the site: fill, peat, beach deposits and glacial deposits. The fill generally consists of very loose to medium dense sand with varying amounts of silt and gravel. Brick and concrete debris, charcoal, and wood debris were encountered within the fill in some of the borings. The fill ranges in depth from about 7.5 to 17 feet below the ground surface (bgs). Very soft to medium stiff peat and organic silt was observed below the fill in some of the borings and was generally 2.5 to 5 feet thick. Beach deposits consisting of very loose to medium sand with varying amounts of silt and gravel, and very soft to very stiff silt with varying amounts of sand were encountered below/within the peat deposits. The beach deposits generally contained wood debris and were generally 10 to 15 feet thick. Glacially consolidated soils consisting of dense to very dense sand with varying amounts of silt and gravel were encountered below the soils described above and were observed to the depths explored in the borings. Interbedded silt and clay was observed within the glacially consolidated soils.

Perched groundwater within the beach deposits is interpreted by ADAPT Engineering at a depth of 14 feet bgs in both borings completed under the building. Static groundwater was interpreted by others at a depth of approximately 19 feet bgs within the vicinity of the site.

REVIEW COMMENTS

Based on our review of the geotechnical report completed by ADAPT Engineering as well as other borings logs in the site vicinity, we have the following comments:

1. We are in general agreement with the subsurface characterization provided by ADAPT Engineering where the borings were drilled at the northern end of the building. We reviewed additional existing geotechnical information prepared by Metropolitan Engineers (1966) and Shannon & Wilson (2004). Site plans and selected borings from these projects are presented as attachments to this letter. Existing boring logs (DT-201 and B-22) are available near the southern end of the Metropole Building. The additional borings demonstrate that the contact to dense to very dense glacially consolidated soils ranges up to 37.5 feet below 2nd Avenue South grades. We recommend that the unbonded micropile length extend to at least 37.5 feet below 2nd Avenue South grades for P1 through P8 in the south part of the building. Alternatively, additional explorations may be completed to assess the depth to dense to very dense glacially consolidated soils.
2. The perched groundwater interpreted by ADAPT Engineering is likely the static groundwater table based on our experience at other sites within the project vicinity as well as boring logs completed by others within the site vicinity. Based on project and construction experience in the Pioneer Square area, the groundwater table ranges from Elevation 16 to 20 feet, which is about 15 to 20 feet below site grades.
3. The recommendations for temporary drainage, dewatering and subgrade preparation are generally appropriate for the site. For structural fill placed below slabs on grade, we recommend specifying Mineral Aggregate Type 2 or Type 17 (1¼-inch minus crushed rock or bank run gravel), City of Seattle Standard Specification 9-03.14
4. The City of Seattle intends to adopt the 2018 International Building Code on March 15, 2021. Given that the project was submitted for permit before this date, the 2012/2015 seismic design parameters provided in the geotechnical report are appropriate.
5. The drainage systems recommendations provided by ADAPT Engineering are appropriate for this project.

6. We recommend designing 7-inch-diameter micropiles for an allowable axial capacity of 3.5 kips per foot in the dense to very dense glacially consolidated soils below a depth of 20 feet. The micropiles should be embedded deep enough to achieve the required allowable axial capacity. Micropile installation should be observed by a representative of GeoEngineers.
7. Driven steel pipe pile recommendations are provided below.
8. The recommendations provided by ADAPT Engineering for structural fill materials and placement are appropriate for the project, with the exception that structural fill used for slab/footing subgrade should be compacted to 95 percent of the maximum dry density (MDD) in general accordance with ASTM International (ASTM) D 1557 test procedure. Structural fill placed below pavement and hardscapes on the site should be compacted to 95 percent of the MDD (ASTM D 1557) within the upper 2 feet, and may be compacted to 90 percent below the upper 2 feet. The City of Seattle requires that structural fill placed in the City Right-of-Way be compacted to 95 percent of the MDD (ASTM D 1557).

DRIVEN STEEL PIPE PILE RECOMMENDATIONS

We understand that 3-inch-diameter driven steel pipe piles will be used for support of new foundations that will be constructed as part of this project. The pipe pile spacing should be determined by the project structural engineer. The pipe piles should be connected with a grade beam to help transfer loads between adjacent piles, as needed.

Steel pipe piles should be installed using pneumatic impact equipment capable of penetrating a sufficient depth to develop the design loads. McDowell Northwest Pile King of Kent, Washington has equipment capable of installing this type of pile. We recommend that the 3-inch-diameter pipe piles be designed for a maximum allowable axial capacity of 12 kips. This load was evaluated based on Allowable Stress Design (ASD), and is for combined dead plus long-term live loads and may be increased by one-third when considering design loads of short duration such as seismic forces. The allowable capacity is based on the strength of the supporting soils and includes a factor of safety of 3 for end bearing and 2 for shaft friction. The capacity applies to single piles. If piles are spaced at least three pile diameter on center, as recommended, no reduction of axial capacity for group action is needed, in our opinion.

The piles should be driven until practical refusal criteria is met to develop the required capacity. We anticipate the pipe piles will be driven at least 25 feet before achieving the required capacity based on the refusal criteria. The piles may be driven open-ended. We recommend that static load tests be completed in accordance with ASTM on a minimum of three percent of installed piles, up to five piles maximum, to verify actual capacity.

Typical refusal criteria for 3-inch steel pipe piles consists of less than 1 inch of penetration after 30 seconds with a 400-pound hammer (TB-125), after 10 seconds with a 850-pound hammer (TB-325), or after 2 seconds with a 2,000-pound hammer (TB-725). The practical refusal criteria depend on the hammer weight and model selected by the contractor and should be reviewed and approved by GeoEngineers prior to construction. Pile installation should be observed by a representative of GeoEngineers. Higher noise levels and vibrations during pile driving to install the steel pipe piles should be evaluated with respect to other building operations or foundations that may be sensitive to these impacts during foundation construction, if needed.

The structural characteristics of pile materials and structural connections may impose limitations on pile capacities and should be evaluated by the structural engineer.

We estimate that the post-construction settlement of pile foundations, designed and installed as recommended, will be on the order of ½-inch or less. Maximum differential settlement should be less than about one-half of the post-construction settlement. Most of this settlement will occur rapidly as loads are applied.

REFERENCES

Metropolitan Engineers, 1966, "Final Report, Subsurface Investigation, Proposed Second Avenue Tunnel, Seattle, Washington."

Shannon and Wilson, 2004, "Report Addendum No. 095-1, Geotechnical Data Report, Seattle Monorail Project, Seattle, Washington."

We trust that this letter meets your present needs. If you have any questions or need additional clarification, please call.

Sincerely,
GeoEngineers, Inc.

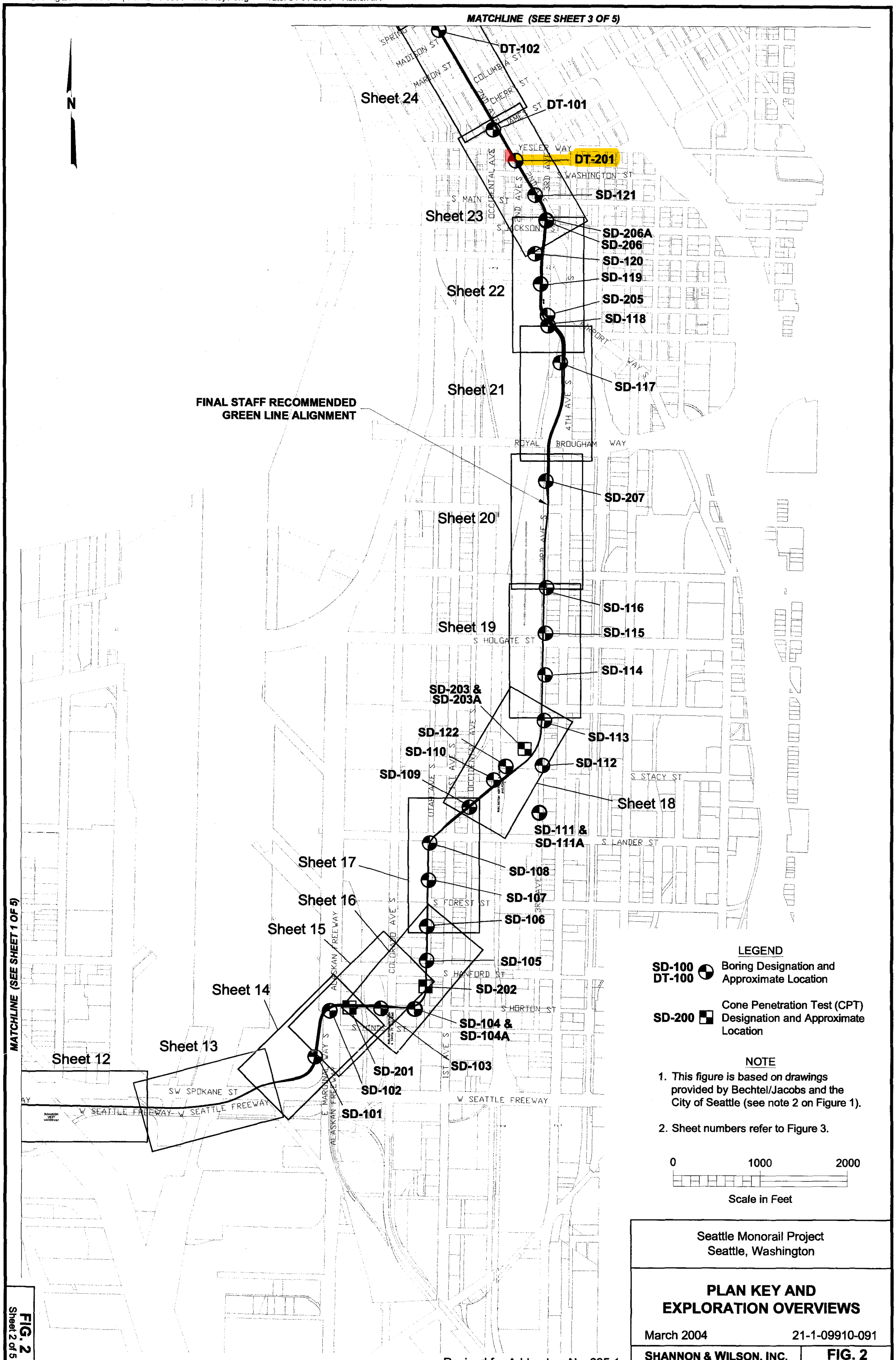
Kyle M. Smith, PE
Geotechnical Engineer

Robert C. Metcalfe, PE, LEG
Principal

CWM:KMS:RCM:nld

Attachments:
Additional Site Plans and Exploration Logs

File: I:\Drafting\211\09910-001\plans\21-1-09910 Vics-Keys.dwg Date: 04-01-2004 Author: LR



LEGEND

SD-100 / DT-100 — Boring Designation and Approximate Location

SD-200 — Cone Penetration Test (CPT) Designation and Approximate Location

NOTE

1. This figure is based on drawings provided by Bechtel/Jacobs and the City of Seattle (see note 2 on Figure 1).
2. Sheet numbers refer to Figure 3.

0 1000 2000

Scale in Feet

Revised for Addendum No. 095-1

Seattle Monorail Project
Seattle, Washington

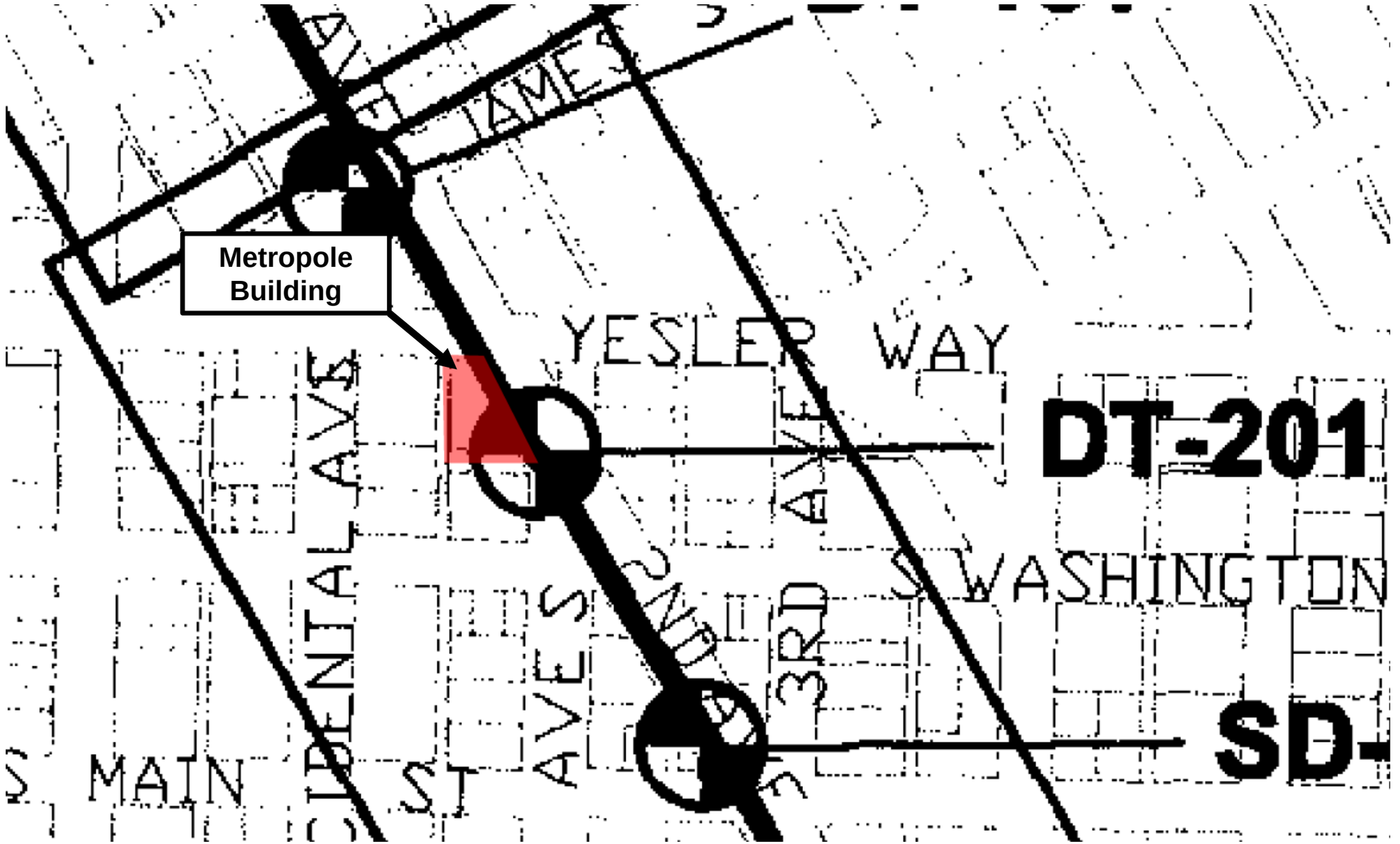
PLAN KEY AND EXPLORATION OVERVIEWS

March 2004 21-1-09910-091

SHANNON & WILSON, INC.
Geotechnical and Environmental Consultants

FIG. 2
Sheet 2 of 5

FIG. 2
Sheet 2 of 5



Notes:

1. The locations of all features shown are approximate.
2. This drawing is for information purposes. It is intended to assist in showing features discussed in an attached document. GeoEngineers, Inc. cannot guarantee the accuracy and content of electronic files. The master file is stored by GeoEngineers, Inc. and will serve as the official record of this communication.

Data Source: Shannon and Wilson, 2004, "Report Addendum No. 095-1, Geotechnical Data Report, Seattle Monorail Project, Seattle, Washington."

N W E S

Not to Scale

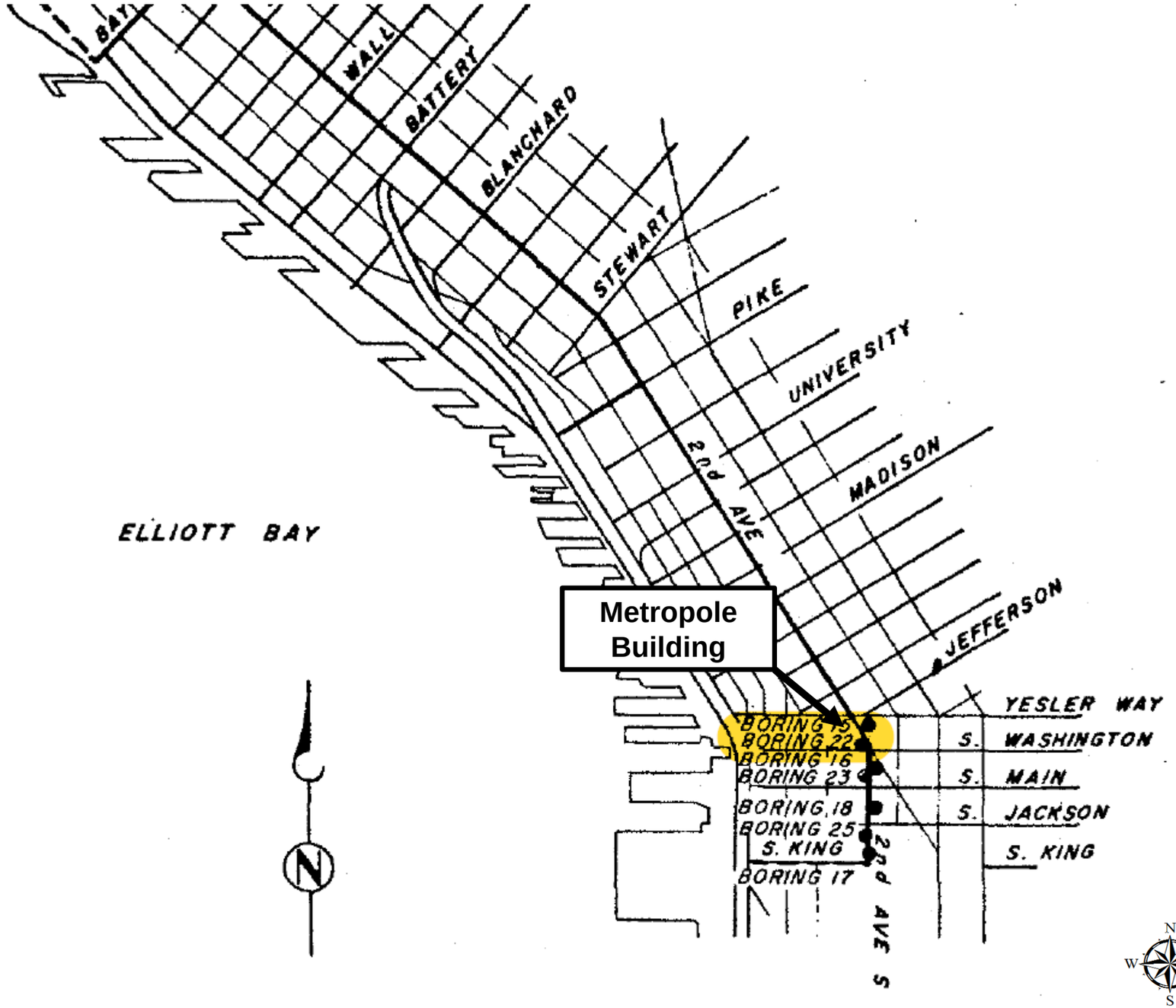
2004 Shannon and Wilson Site Plan

Metropole Building Renovation
Seattle, Washington

GeoEngineers

Figure 1

24937-001-00 Date Exported: 12/23/20



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Data Source: Metropolitan Engineers, 1966, "Final Report, Subsurface Investigation, Proposed Second Avenue Tunnel, Seattle, Washington."

Not to Scale

1966 Metropolitan Engineers Site Plan

Metropole Building Renovation
Seattle, Washington

GeoEngineers

Figure 2

24937-001-00 Date Exported: 12/23/20

SOIL DESCRIPTION

Coordinates: N: 222,934 E: 1,270,776
Surface Elevation: 34.1 Ft. (NAVD-88)

16" CONCRETE, 8" Base/subbase.

Loose to medium dense, gray-brown to red-brown, slightly clayey, silty SAND to gravelly, silty SAND; moist to wet; abundant brick, charcoal, and wood; (Hf) SM.

2.0

Loose to very dense, gray-brown to blue-gray, slightly gravelly, silty, clayey SAND to silty, clayey, sandy GRAVEL; moist to wet; scattered wood; (Ha/Hf) SC/GC.

17.0

Medium dense, gray, slightly silty to silty, fine to medium SAND; wet; scattered gravelly layers and layers of volcanic ash; (Ha) SP-SM/SM.

22.5

Hard, gray-brown, organic, clayey SILT to dense, light-gray, slightly sandy SILT, trace of clay; moist; scattered layers of hard, organic silt to peat; (Qpnl) ML/OL.

37.5

Very dense, dark gray-brown, silty, fine SAND to slightly silty, fine to medium SAND, generally cleaner with depth; wet; (Qpnf) SP-SM/SM.

52.0

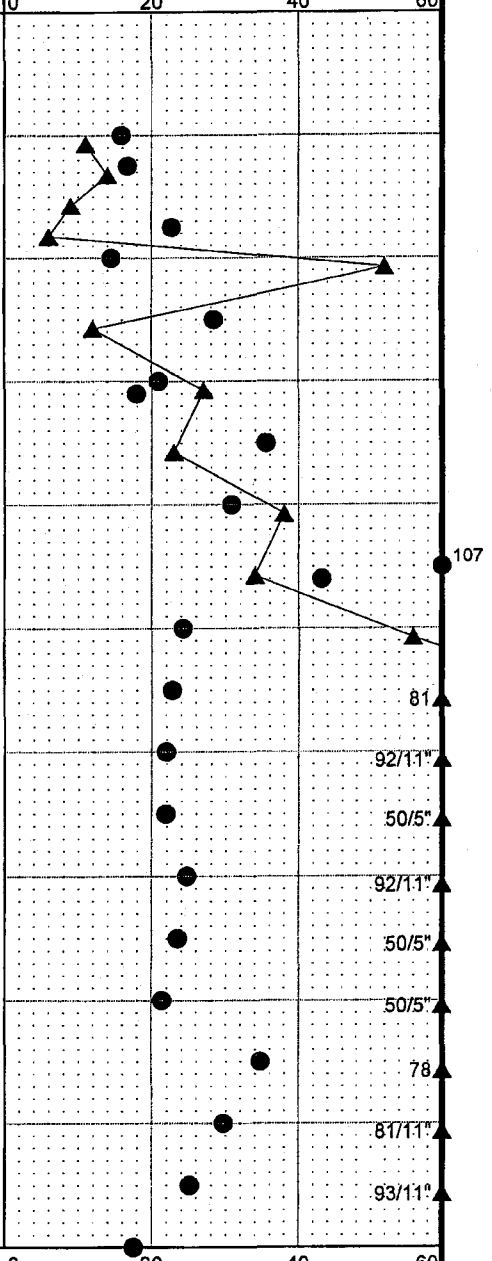
- wood at 85 feet

88.0

Very dense, gray, silty, fine and fine to medium SAND and hard, slightly clayey to clayey, fine sandy SILT; moist to wet; (Qpnl) SM/ML.

CONTINUED NEXT SHEET

Depth, Ft.	Symbol	PID, ppm	Samples	Ground Water	Depth, Ft.
2.0		0	1		
		0	2		
		1	3		
17.0		0.6	4	12/17/2003	
		0	5		
22.5		0.1	6		
		0	7		
37.5		0	8		
		0.1	9		
		0.2	10		
52.0		0.1	11		
		0.3	12		
			13		
			14		
			15		
			16		
			17		
88.0			18		
			19		
			20		



LEGEND

- * Sample Not Recovered
- Standard Penetration Test
- Piezometer Screen and Sand Filter
- Bentonite-Cement Grout
- Bentonite Chips/Pellets
- Bentonite Grout
- Ground Water Level in Well
- ● % Water Content
- Plastic Limit ⊢●⊣ Liquid Limit
- Natural Water Content

NOTES

1. The boring was performed using hollow stem auger drilling methods.
2. The stratification lines represent the approximate boundaries between soil types, and the transition may be gradual.
3. The discussion in the text of this report is necessary for a proper understanding of the nature of the subsurface materials.
4. Groundwater level, if indicated above, is for the date specified and may vary.
5. Refer to KEY for explanation of symbols, codes and definitions.
6. USCS designation is based on visual-manual classification and selected lab testing.

Seattle Monorail Project
Seattle, Washington

LOG OF BORING DT-201

December 2003 21-1-09910-091

SHANNON & WILSON, INC.
Geotechnical and Environmental Consultants

FIG. A.3-6
Sheet 1 of 2

REV 3

MASTER LOG2 21-09910.GPJ SHAN WIL.GDT 12/19/03 Log: JJ Rev: WDN Typ: LKD

SOIL DESCRIPTION

Coordinates: N: 222,934 E: 1,270,776
Surface Elevation: 34.1 Ft. (NAVD-88)

Very dense, silty SAND and sandy SILT; (Qpnl) SM/ML (cont.)

Interbedded, hard, gray, slightly fine sandy, silty CLAY and slightly clayey, fine sandy SILT; moist; scattered to abundant organics and plant fragments; (Qpnl) ML/CL.

102.5

Very dense, gray, slightly silty, fine SAND, trace of fine gravel; wet; (Qpnf) SP-SM.

117.5

120.8

BOTTOM OF BORING
COMPLETED 10/30/2003

Note: Soil descriptions and PID readings above 8.3 feet are based on observations and measurements made during vacuum excavation.

Depth, Ft. | Symbol | PID, ppm | Samples | Ground Water | Depth, Ft.

Standard Penetration Resistance
(140 lb. weight, 30-inch drop)
▲ Blows per foot

0 20 40 60

21 22 23 24 25

50/4"
92/10"
78
94/10"
50/3"

110 120 130 140 150 160 170 180 190

0 20 40 60

● % Water Content
Plastic Limit ⊢●⊣ Liquid Limit
Natural Water Content

LEGEND

* Sample Not Recovered
I Standard Penetration Test

Piezometer Screen and Sand Filter
Bentonite-Cement Grout
Bentonite Chips/Pellets
Bentonite Grout
Ground Water Level in Well

NOTES

1. The boring was performed using hollow stem auger drilling methods.
2. The stratification lines represent the approximate boundaries between soil types, and the transition may be gradual.
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Seattle Monorail Project
Seattle, Washington

LOG OF BORING DT-201

December 2003 21-1-09910-091

SHANNON & WILSON, INC.
Geotechnical and Environmental Consultants

FIG. A.3-6
Sheet 2 of 2

REV 3

MASTER LOG2 21-09910.GPJ SHAN WIL.GDT 12/19/03 Log: JJ Rev: WDN Typ: LKD

ELEVATION - FT.

300
250
200
150
100
MSL
50
0

Washington St

Main St

BORING 15
LOCATION STA 176+10
SEE PLATE 4

1889(?) GROUND SURFACE FROM CITY ENGINEERS RECORDS

BORING 22
LOCATION STA 177+40
(10' RT)
ELEVATION 130
DRILLING DATE 3-14-66

BROWN-GRAY SILTY SAND, GRAVEL, WOOD (FILL)
SEE NOTE 1
SEE NOTE 2
SEE NOTE 3
DRILLS EASIER - SEEPAGE ?
GRAY-BROWN SILT WITH DARK GRAY FINE SAND LENSES, SOME ORGANIC MATTER - FIRM

NOTES:
1. GREEN-GRAY & RUSTY VERY FINE SANDY SILT AND SILTY FINE SAND
2. GRAY SILTY VERY FINE-FINE SAND - DENSE CONTAINS SAND LENSES, SOME GRAVEL
3. GRAY SANDY SILTY GRAVEL, SILTY VERY FINE-FINE SAND & PEBBLY CLAY (CLAY TILL) - DENSE, FIRM
4. WATER LEVEL NOT RECORDED. BORING BACKFILLED W/ PEA GRAVEL

BORING 16
LOCATION STA 179+25
(17' LT)
ELEVATION 126.5
DRILLING DATE 2-15 TO 17-66

GREENISH-BROWN SILTY FINE SAND WITH GRAVEL & LAYERS OF SILT & SANDY SILT (FILL)
CONTAINS WOOD & BRICK - MODERATELY FIRM
SAWDUST - SEE NOTE 1
SEE NOTE 2
GREENISH-GRAY GRAVELLY SANDY SILT & VERY FINE SILTY SAND (TILL) - DENSE & COMPACT
DARK GRAY TO GREEN SAND & GRAVEL - CLEAN, DENSE
SEE NOTE 3
SEE NOTE 4
LAMINATED GRAY TO BROWN ORGANIC SILT & FINE TO VERY FINE SAND

NOTES:
1. CASING & BORING AT EL. 105 OVERNIGHT WL AT EL. 109
2. SILTY PEAT, DECOMPOSED WOOD, SOME GRAY FINE TO MED. SILTY SAND LAYERS, SOME SMALL GRAVEL - SOFT TO VERY SOFT (TIDE MARSH DEPOSIT ?)
3. BROWN ORGANIC SILT W/ VERY THIN PEAT LAMINATIONS - VERY COMPACT
4. CONTAINS ALT'G LAMINATIONS OF GRAY MEDIUM SAND W/ SMALL GRAVEL CASING AT EL. 76, BORING AT EL. 70, OVERNIGHT WL AT EL. 114
5. CASING REMOVED AND BORING BACKFILLED WITH PEA GRAVEL

BORING 23
LOCATION STA 181+00
(6' RT)
ELEVATION 125
DRILLING DATE 3-17, 18-66

CONCRETE, WOOD, CONC.
BROWN, GRAY & GREEN-GRAY SILTY SAND, SILT & SILTY SAND & GRAVEL (FILL)
AFTER SAMPLE, WL AT EL 104
LAYERED GRAY FINE-MED. SAND, BROWN PEATY SAND, PEAT & PEATY SILT WITH WOOD - MODERATELY FIRM TO FIRM
GREEN-GRAY & GRAY FINE-MED. SAND WITH SILTY SAND LAYERS - DENSE
BORING BACKFILLED WITH PEA GRAVEL

BORING 18
LOCATION STA 183
(17' LT)
ELEVATION 124
DRILLING DATE 2-24

GRAY TO BROWN SAN
WITH GRAVEL - SOME
SILT LAYERS (FILL
SEE NOTE 1
SEE NOTE 2
SEE NOTE 3
LOOSE
SEE NOTE 4
SEE NOTE 5
SEE NOTE 6
GRAY TO GREYISH-
SILTY FINE SAND -
SMALL GRAVEL
SEE NOTE 7
SEE NOTE 8
SEE NOTE 9
SEE NOTE 10
FINE TO VERY FI
LAMINATIONS

NOTES:
1. GRADES TO GRAYISH-G
WITH BROWN SILTY FI
LAYERS. CONTAINS C
& WOOD FRAGMENTS
2. PARTIALLY DECOMPO
SAWDUST & WOOD FRA
3. GRAY GRAVELLY SILTY
SOME WOOD (CINDE
4. GRADES LITTLE OR NO
(BEACH DEPOSIT ?)
CASING AT EL. 94, BO
OVERNIGHT WL AT EL.
5. BROWN PEATY SILT
WOOD FRAGMENTS
6. MIXED MED. TO COA
SILTY PEAT & PART
DECOMPOSED WOOD
7. OCCASIONAL SMALL
SOME FINE TO VERY
8. BROWN TO BLACK FINE
PUMICE - SOME SILT -
SMALL ORGANIC MA
9. CONTAINS SILT LAY

STATION
175+00
180+00